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Dynamic properties of earth-core Italian dams from in-situ and laboratory tests

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Seismic assessment of existing dams in Italy

- ✓ More than 500 dams (≅ 170 embk dams) are located in the Italian territory, majority of which constructed between 1950-1970:
- most dams are placed in zones with a degree of seismic hazard that was generally underestimated at the time of their construction as compared with the new national seismic hazard map;
- ✓ seismic design was based on the "classical" pseudo-static approach;
- ✓ a new standard for dam design and evaluation was issued in 2014 (pseudo-static approach no ore allowed; dynamic deformation analysis requested)

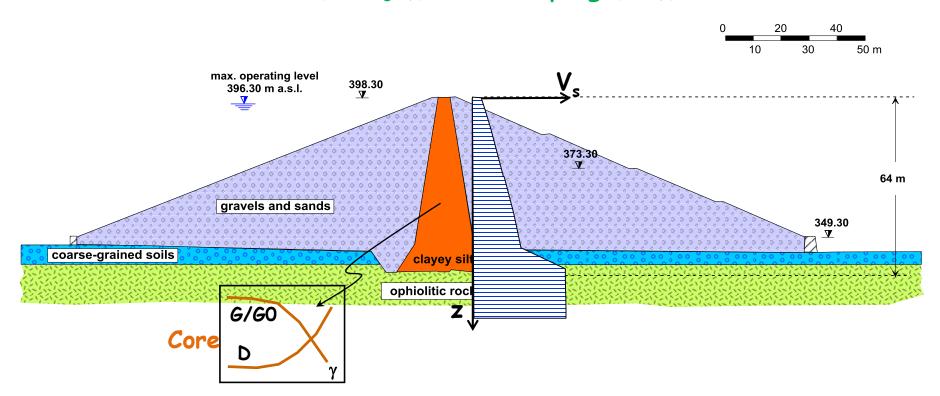
Safety of large dams needs to be reassessed

Dynamic analyses for seismic assessment

Dynamic analyses requires proper characterization of material dam behavior.

Fundamental input parameters for core material:

- ✓ Vs (G_0) profile
- ✓ modulus reduction $(G/G_0-\gamma)$ and damping $(D-\gamma)$ curves



Dynamic properties of core materials

Very limited data:

- V_s (G_0) profile through geophysic
- ✓ <u>invasive</u> (DH, CH, SCPT, su methods are generally prohibit leakage);
- non-invasive surface seismic methods (e.g., MASW)
 applicability of these techniques is restricted by unfavourable testing conditions

Nonlinear stiffness and damping curves

✓ laboratory testing on undisturbed samples seldom available

V_s (G_0) profile in the core of the dams

Empirical correlations used for core material

Mostly used

$$G_0$$
=f (e, σ'_m , OCR, soil type)

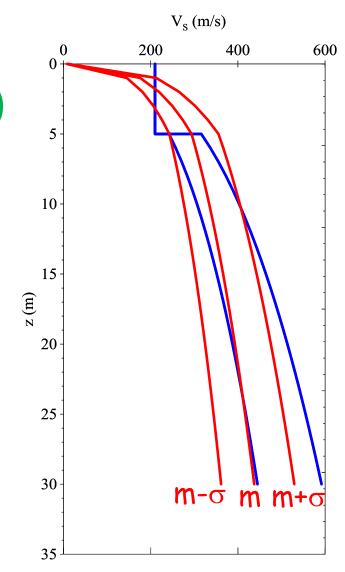
Japan & Korea

Sawada & Takahashi (1975)

Based on Korean data

Park & Kishida (2018)

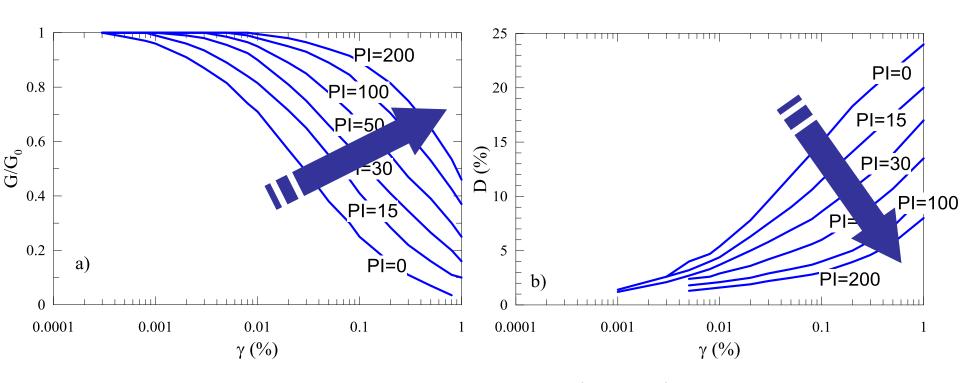
Vs measurements (DH tests) in the core of 21 ECR dams



$G/G_0-\gamma$ and $D-\gamma$ curves

Natural, fine-grained, saturated soils

Effect of plasticity index PI

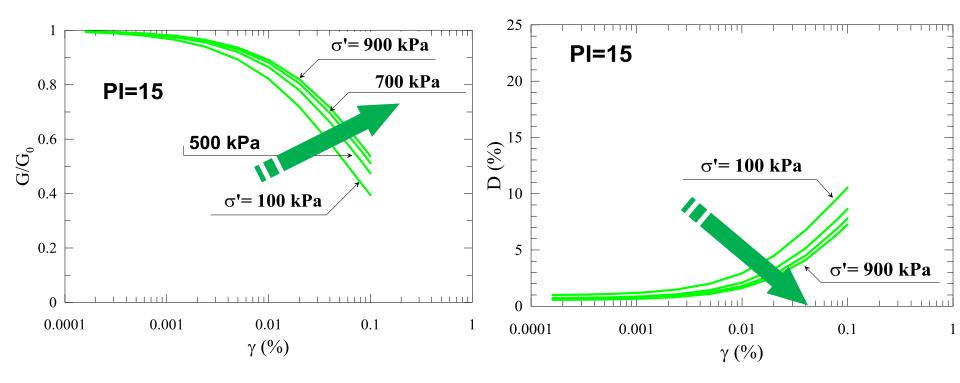


Vucetic & Dobry (1991)

$G/G_0-\gamma$ and $D-\gamma$ curves

Natural, fine-grained, saturated soils

Four-parameter model: most influencial parameters on $G/G_0-\gamma$ and $D-\gamma$ curves are plasticity index and confining presssure

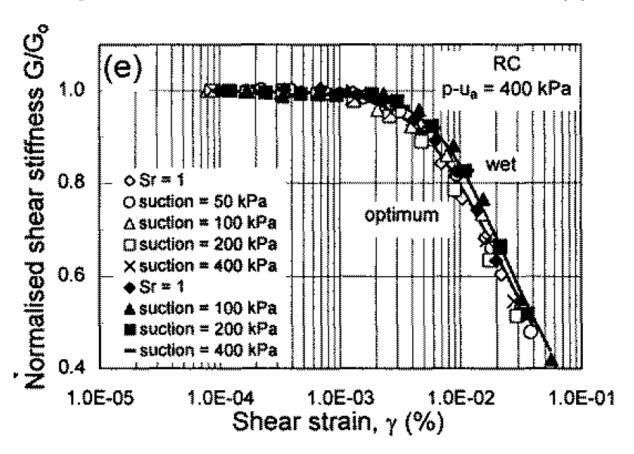


Darendeli (2001)

$G/G_0-\gamma$ curves

Lab-compacted soils

Partially saturated and saturated, <u>Metramo silty clayey</u> sand using a suction-controlled RC/TS apparatus

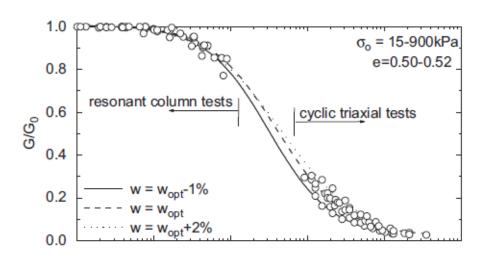


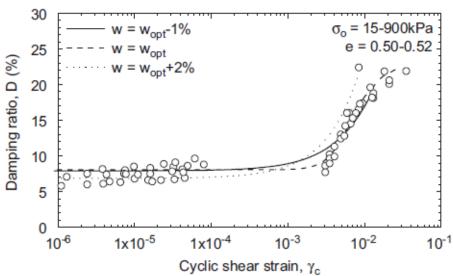
Vinale et al. (2001)

$G/G_0-\gamma$ curves

Lab-compacted soils

- √ sandy-silty clay using RC/TS and CTX apparatus
- ✓ soil compacted at w_{opt} , w_{opt} -1% and w_{opt} +2%
- √ high range of confining stress (15-900 kPa)



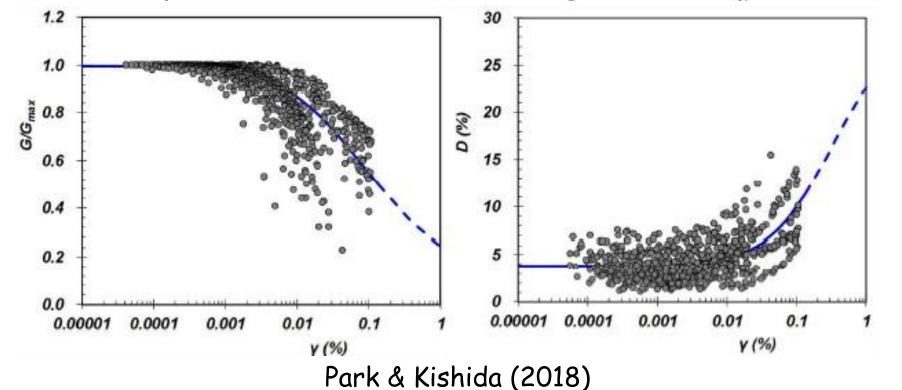


Xenaki & Athanasopoulos (2008)

$G/G_0-\gamma$ and $D-\gamma$ curves

Field-compacted, undisturbed core samples

- ✓ Undisturbed samples from 13 Korean dams
- √ RC tests on 17 undisturbed and 14 reconstituted
- ✓ Plasticity index PI=4-50; confining stress σ'_{m} <400 kPa



Italian earth-core rockfill dams

List of dams examined

#	Dam	Construction period	H _{max} (m)	Reference
1	Angitola (VV)	1960-1966	22.8 22.6	<u>unpublished</u>
2	Bilancino (FI)	1988-1995	42	Mancuso et al. (1993)
3	Camastra (PZ)	1963-1964	54	Pagano et al. (2008)
4	Farneto (CS)	1970-1980	27.7	<u>unpublished</u>
5	Montedoglio (AR)	1977-1986	64.3	Lanzo et al. (2015)
6	Penne (PE)	1966-1969	35.7	<u>unpublished</u>
7	Poggio Cancelli (AQ)	1950-1951	27.3	<u>unpublished</u>
		1964-1971		
8	Polverina (MC)	1963-1967	27.5	<u>unpublished</u>
9	San Pietro (AV)	1958-1964	49	Calabresi et al. (2004)
10	San Pietro in Villa (AR)	1980-1993	6.30	<u>unpublished</u>

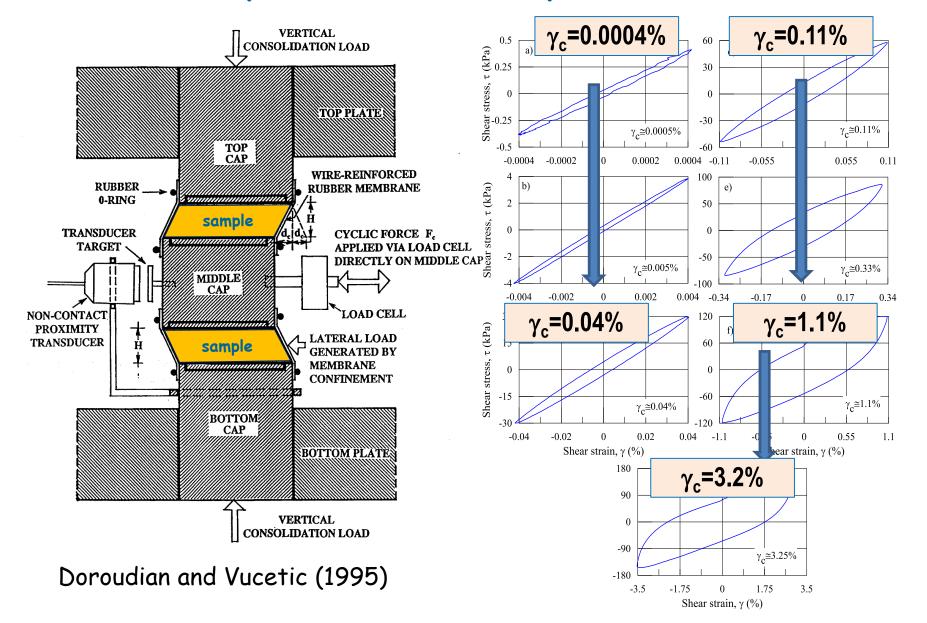
Italian earth-core rockfill dams

Field and laboratory tests

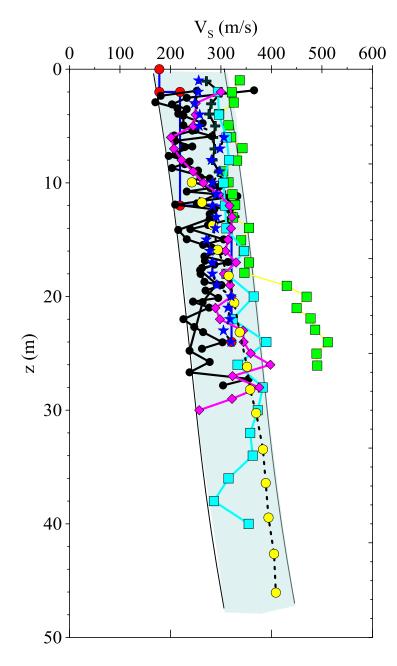
#	Dam	In situ dynamic	Laboratory cyclic/dynamic			
1	Angitola (VV)	DH, SCPT	DSDSS			
2	Bilancino (FI)	DH	RC/TS			
3	Camastra (PZ)	SDMT	-			
4	Farneto (CS)	СН	RC/TS			
5	Montedoglio (AR)	СН	DSDSS			
6	Penne (PE)	-	DSDSS			
7	Poggio Cancelli (AQ)	СН	RC/TS			
8	Polverina (MC)	СН	DSDSS, RC/TS			
9	San Pietro (AV)	СН	RC/TS			
10	San Pietro in Villa (AR)	СН	DSDSS			
In situ tests: DH=Down-Hole, CH=Cross-Hole, SCPT= Seismic cone, SDMT=Seismic dilatometer.						
Cyclic/Dynamic tests: DSDSS (Double Specimen Direct Simple Shear); RC=Resonant Column; TS=Torsional Shear						

The DSDSS device

Double Specimen Direct Simple Shear (DSDSS)



Italian earth-core rockfill dams



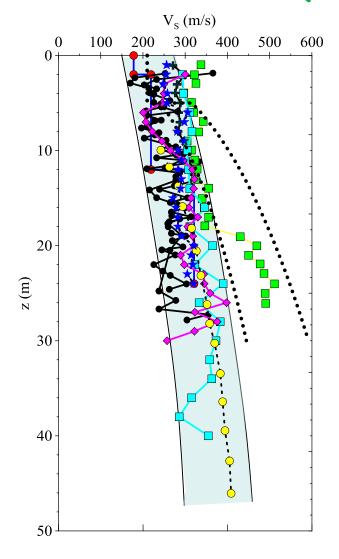
Vs profiles in the core materials

- Angitola DH
- + Bilancino CH
- Camastra SDMT
- Farneto del Principe CH
- Montedoglio CH
- Poggio Cancelli CH
- ★ Polverina CH
- San Pietro CH
- San Pietro in Villa CH

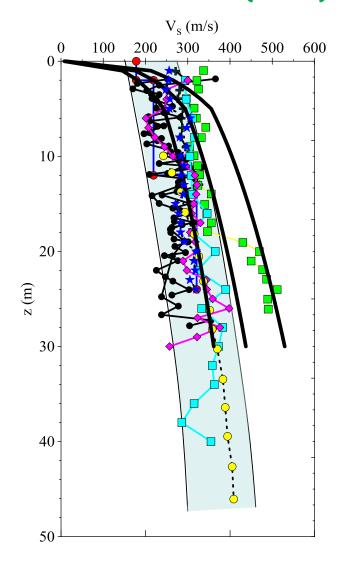
Italian earth-core rockfill (ECR) dams

Italian data vs. empirical correlations

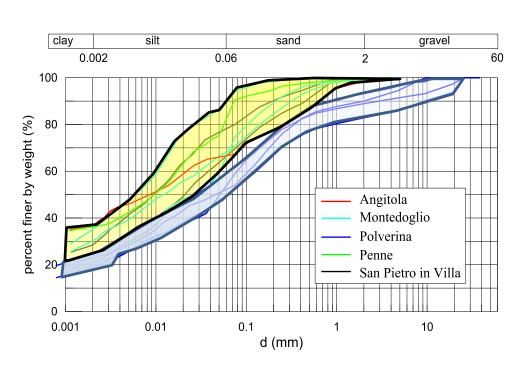
Sawada e Takahashi (1975)

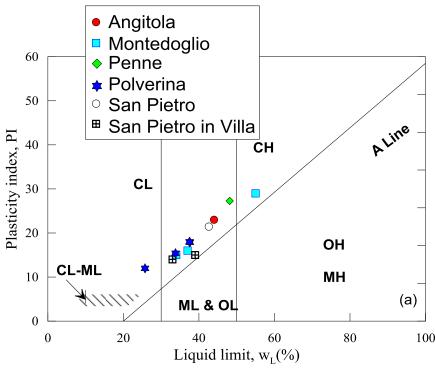


Park e Kishida (2018)



Grain size distributions



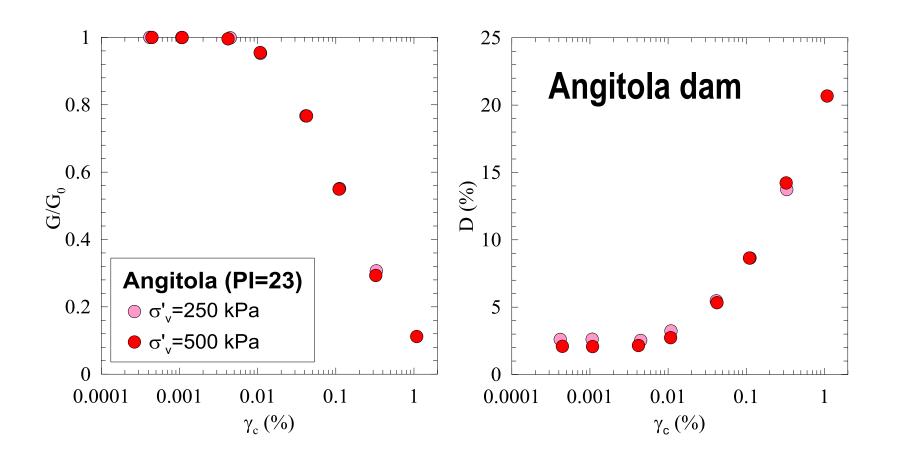


Sandy-clayey silt

PI=12-30

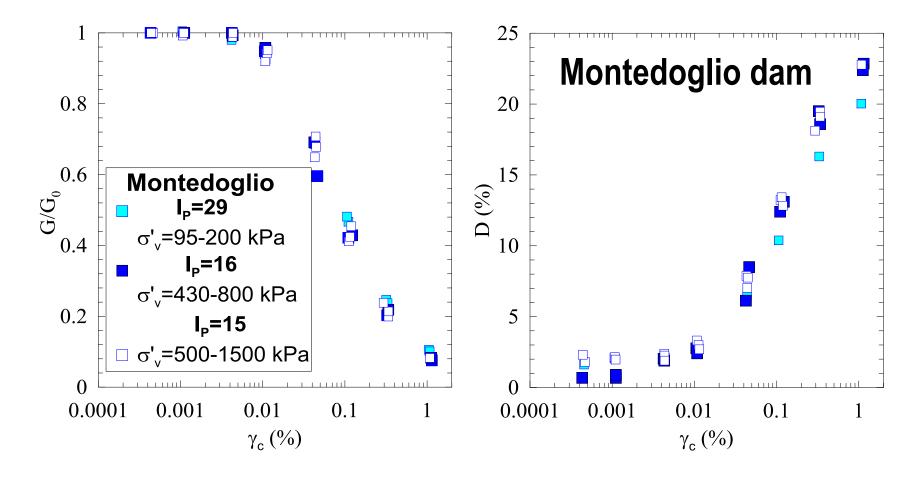
Results of cyclic DSDSS tests

✓ Nonlinear stiffness and damping properties from DSDSS test results on undisturbed core samples (PI=23)



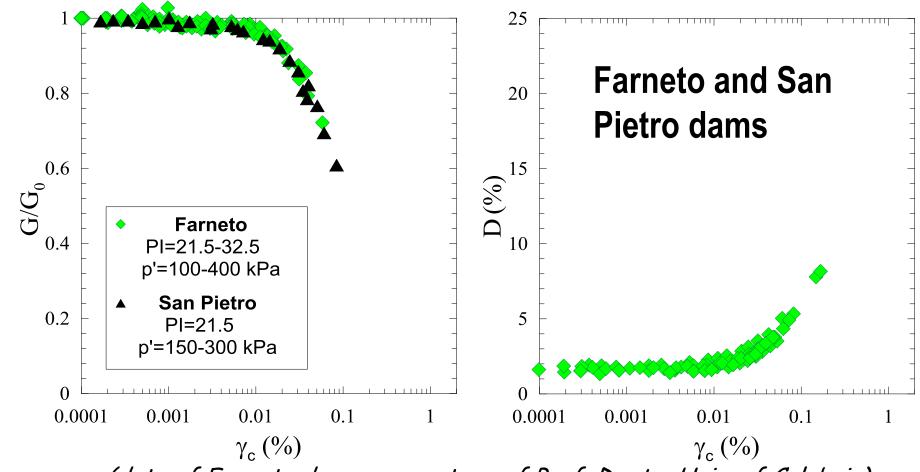
Results of cyclic DSDSS tests

✓ Nonlinear stiffness and damping properties from DSDSS test results on undisturbed core samples (PI=15-29)



Resonant Column (RC) and Torsional Shear (TS) tests

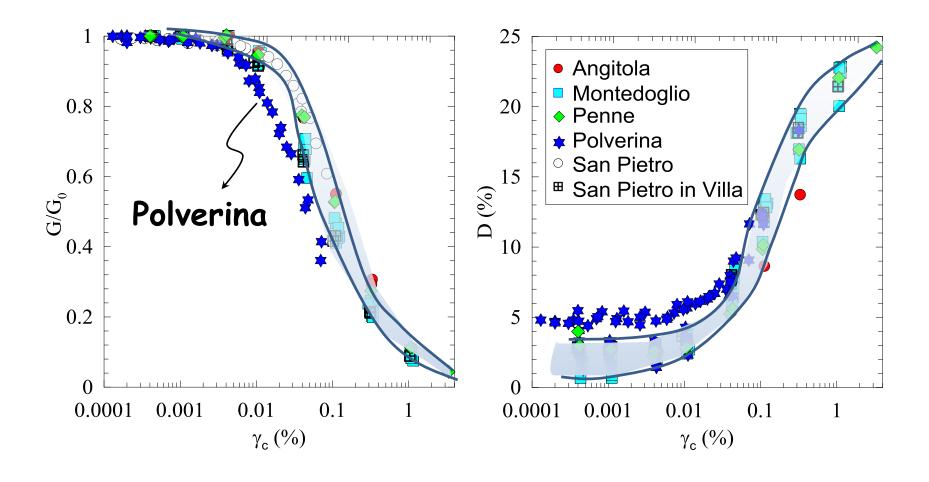
✓ Nonlinear stiffness and damping properties for Farneto (PI=22-32) and San Pietro (PI=22) dams



(data of Farneto dam are courtesy of Prof. Dente, Univ. of Calabria)

Core materials of Italian zoned dams Cyclic and dynamic test results

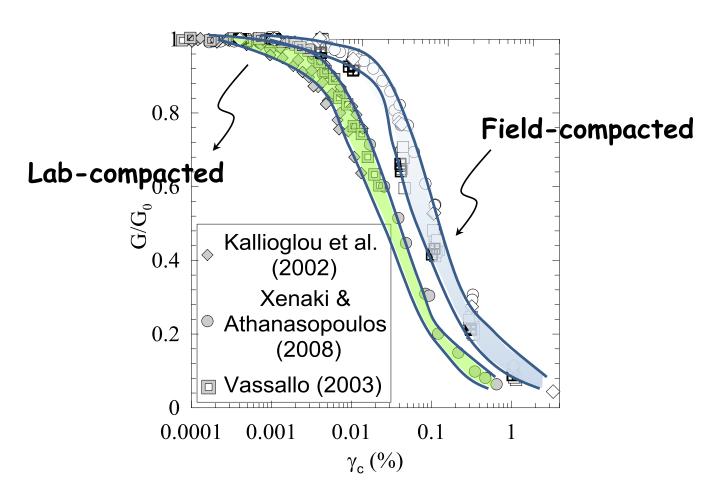
✓ Nonlinear stiffness and damping properties of core material of Italian dams



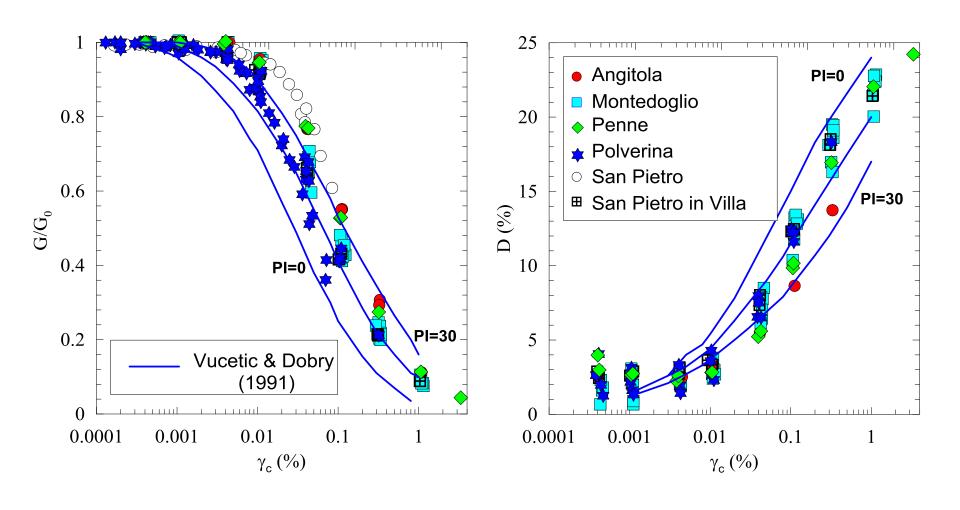
Core materials of Italian zoned dams Nonlinear stiffness and damping properties

✓ Lab-compacted vs field-compacted core materials

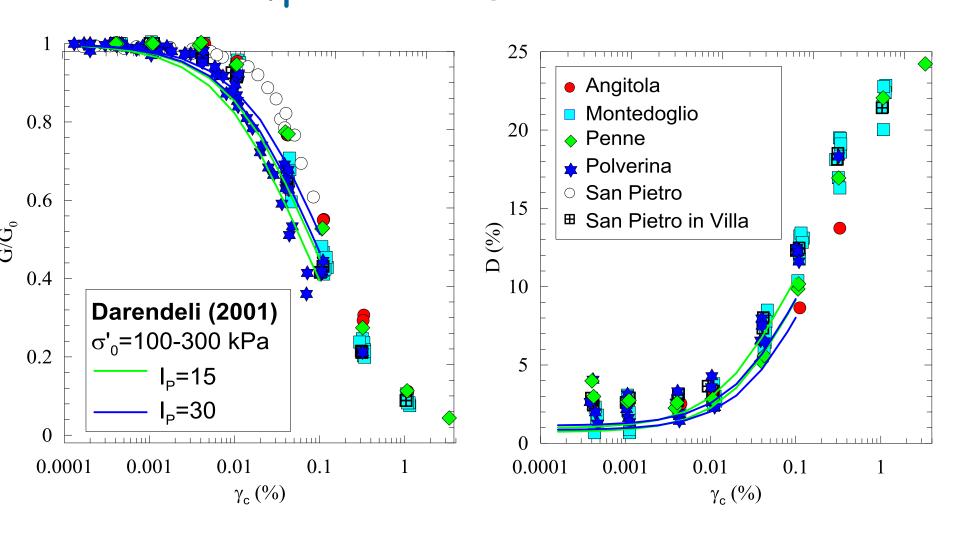
<u>Data with PI=15-30</u>



Results of cyclic and dynamic tests Comparison with Vucetic & Dobry curves



Results of cyclic and dynamic tests Comparison with Darendeli curves



Final remarks

The use of dynamic analyses of embankments dams is increasing significantly in engineering practice, therefore a proper characterization of dynamic properties of dam body is needed. Data on dynamic properties of core materials is extremely limited.

Based on the results on the core of Italian dams:

- √ Vs measured data does not satisfactory agree with empirical correlations
- ✓ $G/G0-\gamma$ and $D-\gamma$ curves from lab tests on undisturbed samples do not follow trends typical of natural saturated fine-grained soils.
- for medium plasticity soils (PI=15-30), there is no significant effect of plasticity index and confining stress
- lab-compacted modulus reduction curves show higher nonlinearity than undisturbed core materials
- the use of predictive models (Vucetic & Dobry and Darendeli) is questionable.

Site-specific laboratory tests are recommended to model the nonlinear deformation behavior of core materials

Thank you!!