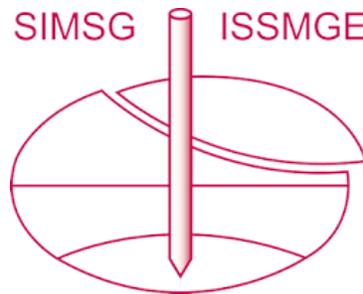


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# Effects of Detailing on the Behavior of Helical Pile-to-Foundation Connections

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**Abstract.** Helical piles present a significant potential to create resilient, durable, and faster-to-construct foundations while ensuring a long-term and maintenance-free service life. To achieve their full potential, it is imperative that helical pile anchorages (i.e., pile-to-foundation connections) perform well without resulting in any cracking to the surrounding concrete. However, there is a lack of research and design guidelines on how to design resilient connections, especially for load conditions that create net uplift and cyclic loads. The objective of this study is to advance the current understanding and quantify the influences of anchorage bracket embedment depth, longitudinal reinforcement ( $\rho_x$ ) percentage, shear span-to-depth ( $a/d$ ) ratio, and the loading conditions on the load, deformation, cracking, and failure behavior of concrete foundations. For this purpose, 81 high-fidelity nonlinear finite element simulations of a pile cap strip are performed under monotonic tension, monotonic compression, and reversed-cyclic loads. The results indicate that the anchorage response may govern the response of the entire foundation system. Connection brackets with low embedment depths have been found to significantly reduce the load capacity of the foundation system, with a failure mode involving extensive anchorage zone cracking. The results also indicate that high  $\rho_x$  percentages and low  $a/d$  ratios should be used, along with higher embedment depths, in order to maximize the load resistance and prevent anchorage cracks for the load conditions involving tensile uplift loads. The research findings have applicability to both helical and micro piles given the fact that they both include similar anchorage conditions.

**Keywords.** Helical piles, helical pile anchorage, parameters, single bracket, longitudinal reinforcement, embedment depth,  $a/d$  ratio.

## 1. Introduction

Many structures are subjected to ‘cyclic loads,’ which involve sequential application of compression and tension loads due to events such as windstorms, earthquakes, or heavy vehicular traffic. Tall and light structures—such as telecommunication or transmission towers, wind turbines, and light-frame steel buildings—are particularly vulnerable to the tensile uplift components of cyclic loads. Helical piles (see Figure 1a) inherently possess high uplift resistances and thus present a significant potential to resilient, durable, and faster-to-construct foundations. To realize the full potential of helical piles, it is imperative that the helical pile anchorages (i.e., pile-to-foundation connections; see typically focus on the behavior of isolated helical piles without considering the influence of the concrete footing [1-8], while the structural studies focus on the footing response

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[9-11]. Helical pile-to-footing connections are commonly designed in practice using ‘rules of thumb’ or simple bearing stress checks [e.g., 12]. Yet the failure of an Olympic-size swimming pool [13] and recent experimental studies [14] have shown this to be an oversight, by demonstrating that pile anchorages may govern the system response.

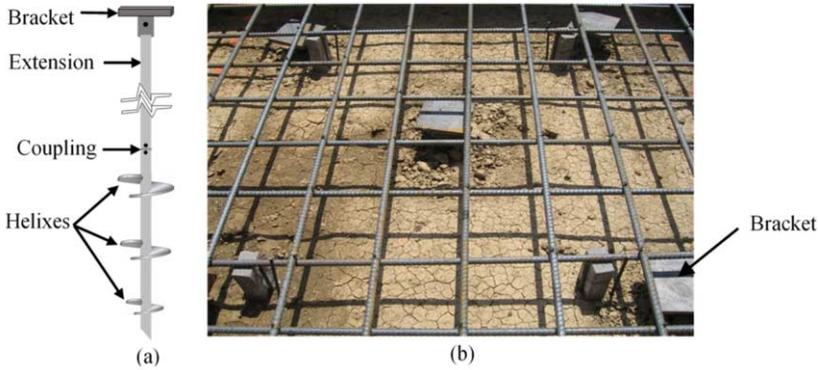


Figure 1. (a) A typical helical pile; (b) Anchorage of helical pile to a pile cap.

## 2. Objectives

The objective of this study is to quantify the influences of different pile anchorage configurations on the holistic behavior of pile caps—in terms of the load, deformation, cracking, and failure response—subjected to monotonic compression, monotonic tension, and reversed-cyclic load conditions.

## 3. Foundation system details

A foundation strip representative of commonly used foundation configurations (e.g., strip footings, grade beams, or a segment of pile caps) is designed, following the CRSI recommendations [15]. The strip is supported by two helical piles to create a one-way stress flow and better isolate the bracket response (see Figure 2).

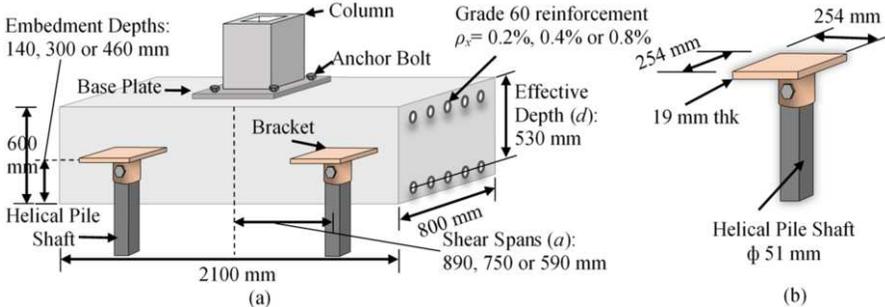


Figure 2. (a) Helical pile anchorage in the pile cap strip; (b) Details of single bracket.

Influencing parameters investigated include: three bracket embedment depths: 460 mm (top), 300 mm (middle), 140 mm (bottom); three longitudinal reinforcement percentages ( $\rho_x$ ): minimum 0.2% (5-#5 rebars), 0.4% (7-#6 rebars) and 0.8% (10-#7 rebars); and three shear span-to-depth  $a/d$  ratios: 1.68, 1.42 and 1.11 (See Figure 2). When considering three loading conditions,  $3^4=81$  foundation systems are created.

#### 4. Nonlinear finite element modeling

A two-dimensional, continuum-type, plane-stress element is used for the finite element modeling through the computer program VecTor2 [16]. The formulation is based on the Disturbed Stress Field Model [17], which is an extension of the Modified Compression Field Theory (MCFT) [18]. One sample numerical model created is shown in Figure 3a, where the concrete is modeled using a four-noded rectangular element with 8 degrees of freedom (see Figure 3b), and the longitudinal reinforcement is modelled using a two-noded truss bar (see Figure 3c) with 4 degrees of freedom. Table 1 shows the component properties in terms of their strengths and dimensions. VecTor2 incorporates several second-order material behaviors [19, 20] as listed in Table 2, including the cyclic response hysteresis (see Figures 4a and 4b) important for this study. A very fine mesh with a size of 20 mm x 20 mm is used where each helical pile is restrained with four hinges. A displacement-controlled analysis is employed which is advantageous when simulating the post-peak response, ductility, crack patterns, and failure modes. The results of the reversed-cyclic loading are divided into ‘cyclic compression’ and ‘cyclic tension’ to facilitate the comparison with the monotonic loading. *Cyclic tension* represents the uplift effects and *cyclic compression* represents the compressive effect (see Figure 5c).

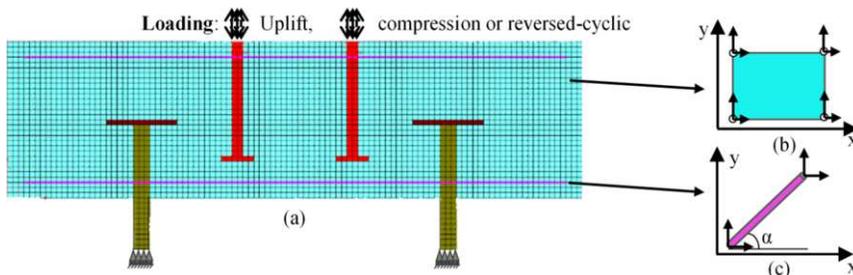


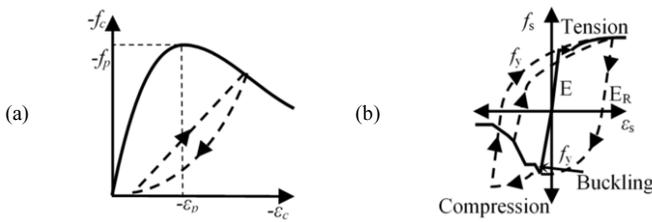
Figure 3. (a) Nonlinear FEM; (b) Plane stress rectangle elements; (c) Truss bar elements.

Table 1. Material properties.

Material	Description	Color	$f_c$ (MPa)	$f_y$ (MPa)	Thickness (mm)
1	Concrete		20.7	-	800
2	Helical Pile		-	552	44
3	Bracket Plate		-	345	260
4	Anchor Bolt		-	724	57
5	Longitudinal reinforcement		-	414	-

**Table 2.** Concrete and reinforcement material models.

Material Behaviour	Default Model
Compressive Base Curve	Hognestad
Compression Post-Peak	Modified Park-Kent
Compression Softening	Vecchio 1992-:
Tension Stiffening	Modified Bentz 2003
Tension Softening	Linear
Confined Strength	Kupler/Richart
Concrete Dilation	Variable-Isotropic
Cracking criterion	Mohr-Coulomb (Stress)
Crack Width Check	Agg/5 Max crack width
Crack Slip	Walraven
Hysteretic Response	Bauchinger Effect (Seckin)
Dowel Action	Tassios (Crack Slip)
Buckling	Akkaya et al. 2019

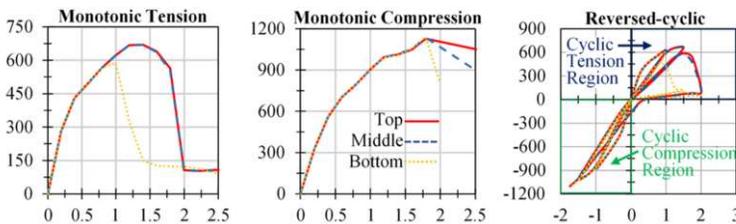


**Figure 4.** (a) Cyclic response of concrete; (b) Hysteretic response ductile steel reinforcement

**5. Results**

The simulation results show that the highest tensile resistance is obtained for the configurations involving higher embedment depths, higher  $\rho_x$  percentages, and lower  $a/d$  ratios. However, the compressive resistance of the foundation was found independent of the embedment depths. Figure 5 shows the load-displacement curves for a sample configuration involving an  $a/d$  ratio of 1.68 and  $\rho_x$  percentage of 0.4%.

Figure 6 shows the changes in load capacities in relation to the parameters investigated for all simulations. For the same color, the dashed and dotted lines lie above the solid lines in Figures 6a and 6b, which shows the increase of load capacity with the increase of embedment depth. The tensile load capacity is increased by an average of 29% when the embedment depth is changed from bottom to middle. In the same figures, the overlapped dashed and dotted lines confirm the similar load capacities for middle and top embedment depths.



**Figure 5.** Load-displacement responses for  $a/d=1.68$  and  $\rho_x=0.4\%$ .

The tensile load capacity increases by an average of 23% and 19% when the  $\rho_x$  is increased from 0.2% to 0.4% and 0.4% to 0.8%, respectively. The tensile load capacity also increases by an average of 18% and 19% when the  $a/d$  ratio is decreased from 1.68 to 1.42 and 1.42 to 1.11, respectively. However, the tensile load capacity does not increase significantly for the bottom embedment depth even if the  $\rho_x$  percentage is increased or  $a/d$  ratio is reduced significantly. In Figures 6a and 6b, for example, the solid red, yellow and blue lines represent the bottom embedment depth; they all have smaller slopes as compared to other lines and are closer to each other. In addition, Figures 7a and 7b shows that the load capacity predictions for the bottom embedment depth are concentrated and do not increase as other embedment depths do.

The overlapping of the same colored lines in Figures 6c and 6d indicate that there is no significant influence of the embedment depth on the compressive resistance. The  $\rho_x$  percentage, on the other end, has a significant influence; the compressive capacity increases by an average of 24% and 26% when  $\rho_x$  is increased from 0.2% to 0.4% and 0.4% to 0.8%, respectively. Similarly, the compressive load capacity increases by an average of 25% when the  $a/d$  is decreased from 1.68 to 1.42 and 1.42 to 1.11.

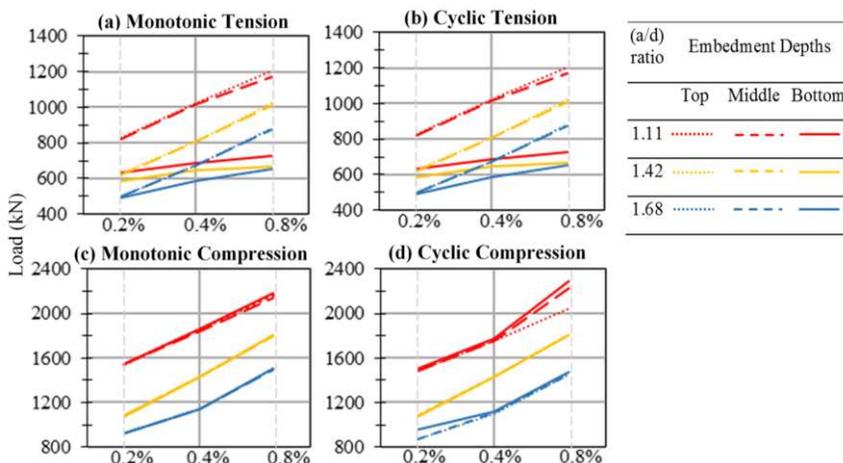


Figure 6. Effect of parameters on the load capacity subjected to different loading.

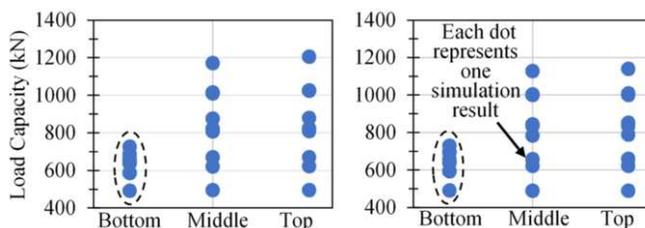
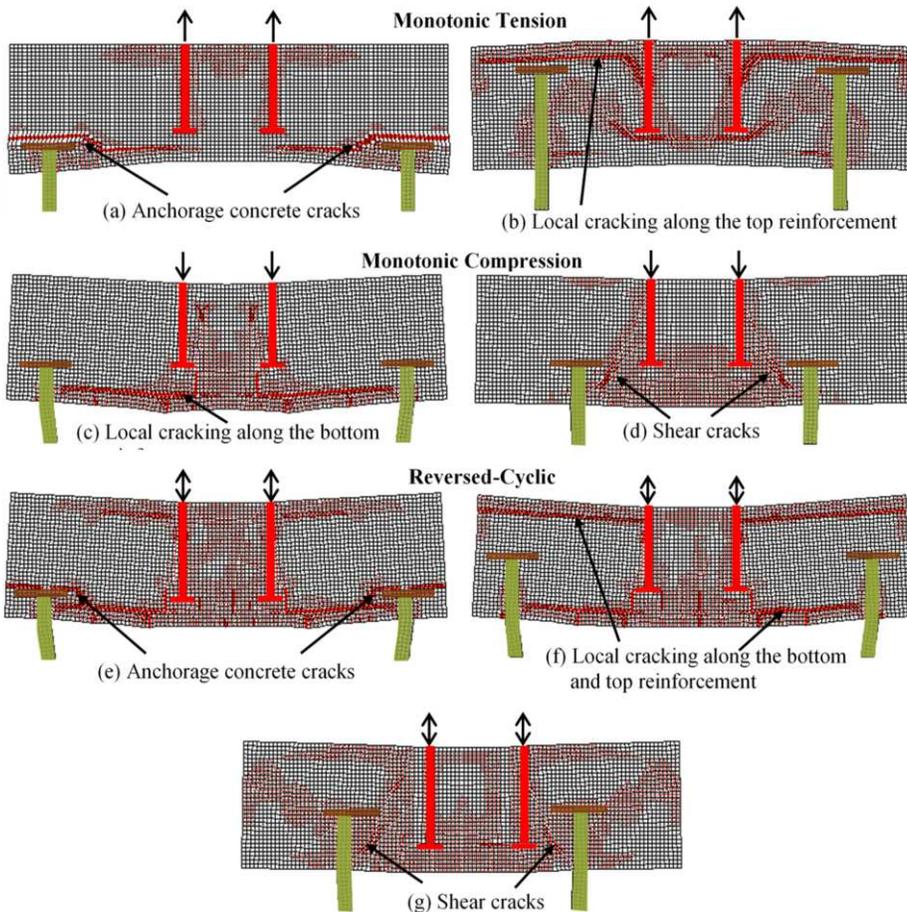


Figure 7. Summary of the ultimate load capacities from 54 simulation results for tension loads.

### 6. Failure modes and crack patterns

For monotonic tension loads, the simulation results indicate anchorage-zone concrete cracks for the bottom embedment depths (see Figure 8a), and local concrete cracking

along the top longitudinal reinforcement (see Figure 8b) for higher embedment depths. For monotonic compression loads, most failure modes are predicted as splitting of the concrete along the bottom reinforcement (see Figure 8c), and shear failures for the lowest  $a/d$  ratio (i.e., 1.11) with  $\rho_x$  of 0.4% or 0.8% (see Figure 8d). For reversed-cyclic loads, simulation results indicate anchorage-zone concrete cracks for the bottom embedment depths (see Figure 8e), and either local concrete cracking along the top and bottom longitudinal reinforcement (see Figure 8f) or shear cracks for a few cases in the lowest  $a/d$  (i.e., 1.11) and highest  $\rho_x$  (i.e., 0.8%) ratios (see Figure 8g) for higher embedment depths. Anchorage concrete cracks decrease the global load resistance of the foundation as compare to middle or top embedment depths. This could be attributed to insufficient bond resistance available in the helical shaft due to their shallow lengths.



**Figure 8.** Failure modes and crack patterns.

## 7. Summary and conclusion

To fill the current knowledge gaps for helical pile anchorages, high-fidelity nonlinear finite element models were created (applicable to any general configuration of helical

piles and anchorages) to numerically investigate the effectiveness of commonly-used single bracket anchorages as well as the influence of embedment depths, longitudinal reinforcement ( $\rho_x$ ) percentages, and shear span-to-depth ( $a/d$ ) ratios. The results support the following conclusions.

#### *Load Capacity for Monotonic and Cyclic Tension Loads*

- The analysis indicates that ‘embedment depth’ is the most influential parameter, which also dictates the effectiveness of ‘ $\rho_x$  percentage’ and ‘ $a/d$  ratio’ on the tensile load resistance.
- The tensile load capacity increases by an average of 29% when the ‘embedment depth’ is changed from bottom to middle.
- The analysis also reveals that there is no difference in tensile load resistance when the ‘embedment depth’ is either at the middle or top.
- The tensile load capacity increases by an average of 20% when  $\rho_x$  is increased from 0.2% to 0.4% or 0.4% to 0.8%.
- The tensile load capacity increases by an average of 19% when  $a/d$  ratio is decreased from 1.68 to 1.42 or 1.42 to 1.11.
- To maximize the tensile load resistance, high  $\rho_x$  percentages and low  $a/d$  ratios should be used, along with middle or top embedment depths.

#### *Load Capacity for Monotonic and Cyclic Compression*

- The ‘ $\rho_x$  percentage’ and ‘ $a/d$  ratio’ influence the compressive load resistance while ‘embedment depth’ makes no influence.
- The compressive load capacity increases by an average of 25% when  $\rho_x$  is increased from 0.2% to 0.4% or 0.4% to 0.8%.
- The compressive load capacity increases by an average of 25% when the  $a/d$  ratio is decreased from 1.68 to 1.42 or 1.42 to 1.11.
- To maximize the compressive load resistance, high ‘ $\rho_x$  percentages’ and low ‘ $a/d$  ratios’ should be used, regardless of the embedment depth.

#### *Failure Modes*

- Anchorage cracks (i.e., splitting concrete cracks around the brackets) for bottom embedment depths subjected to monotonic tension or reversed-cyclic loads are found to significantly decrease the global load resistance of helical pile foundation.

## **Acknowledgements**

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